

Course No. CE 1023

Assignment No.

Date

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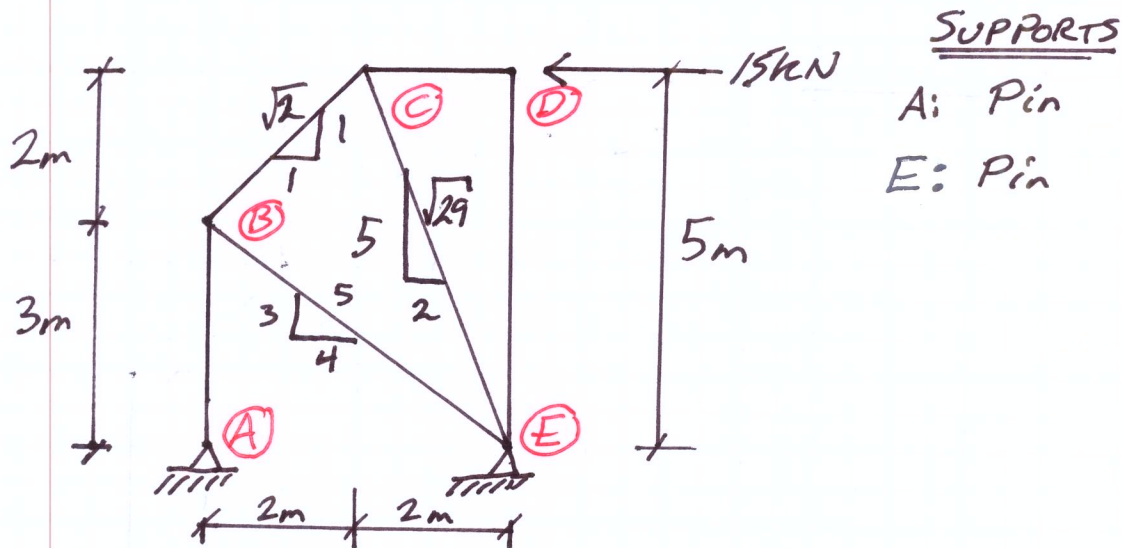
Problem No. Method of Joints 1

By ALAN CLOPP

of

For the truss below:

- Assess determinacy & stability
- Solve for ALL member forces.



• Stability and determinacy

• Determinacy

$$SI = b + r - 2j$$

$$b = 6$$

$$r = 4$$

$$j = 5$$

$$SI = 6 + 4 - 2(5)$$

$$SI = 0$$

∴ determinate

• Stability

- $SI \geq 0$ ✓

- not all reactions are

• parallel ✓

• concurrent ✓

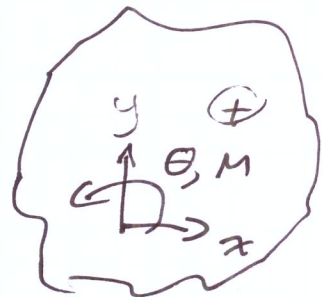
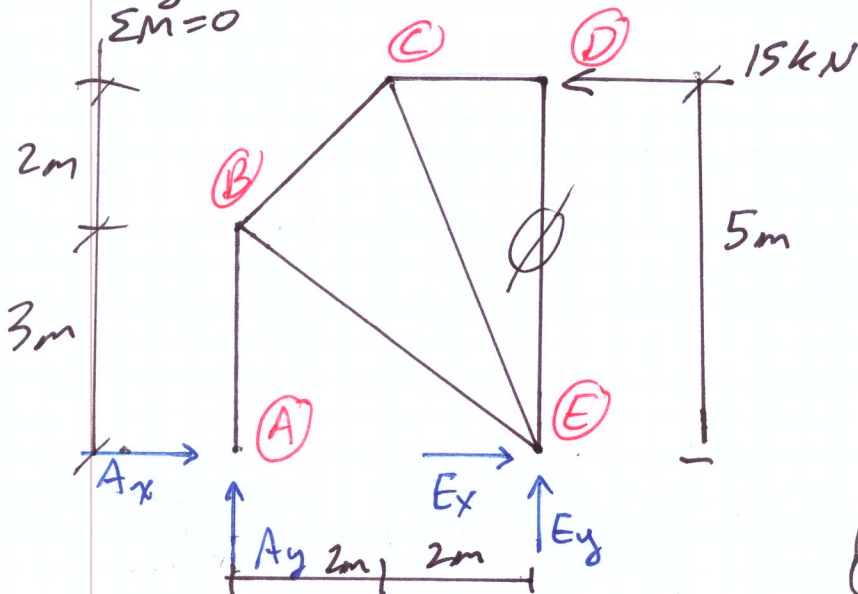
- do not have internal collapse mechanism ✓

∴ stable

• Can we find reactions

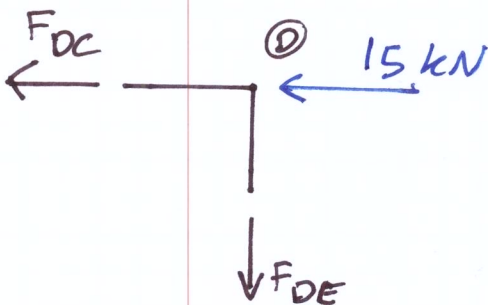
- 4 reactions
 - 3 equations
- } we can not solve for reactions now

$$\begin{aligned} \sum F_x &= 0 \\ \sum F_y &= 0 \\ \sum M &= 0 \end{aligned}$$



Method of Joints

Joint D



$$\sum F_x = 0 \quad -F_{DC} - 15\text{kN} = 0$$

$$F_{DC} = -15\text{kN}$$

F_{DC} is acting to the right
 F_{DC} is in Compression.

$$F_{DC} = 15\text{kN (c)}$$

Case 1

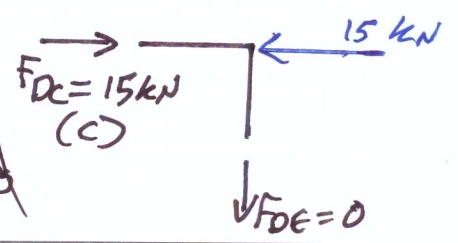
- non-collinear
- no external force or reaction
- Both are zero force

Case 2

- 3 members
- 2 collinear
- no external loads or reactions
- non-collinear mem. is zero-force

$$\sum F_y = 0 \quad F_{DE} = 0$$

F_{DE} is zero-force member



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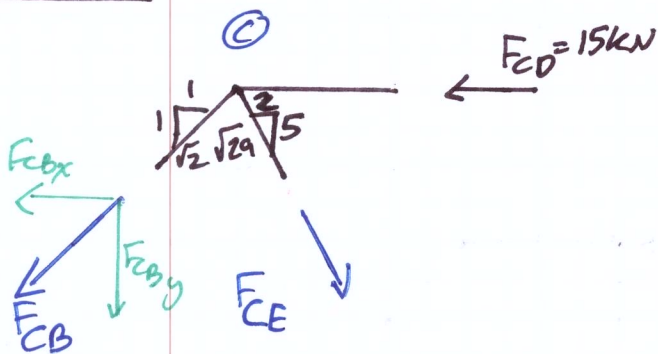
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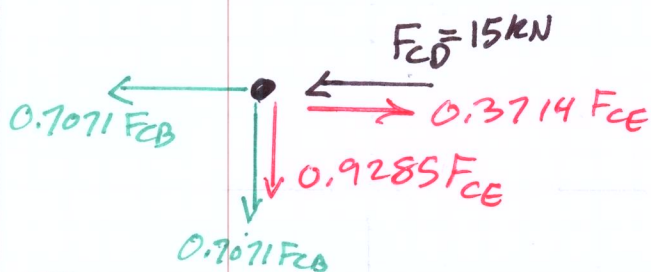
ALAN LLOYDJOINT CComponents of F_{CB} , F_{CE}

$$\underline{F_{CE}} \quad F_{CE_x} = \frac{2}{\sqrt{29}} F_{CE} = 0.3714 F_{CE}$$

$$F_{CE_y} = \frac{5}{\sqrt{29}} F_{CE} = 0.9285 F_{CE}$$

$$\underline{F_{CB}} \quad F_{CB_x} = \frac{F_{CB}}{\sqrt{2}} = 0.7071 F_{CB}$$

$$F_{CB_y} = \frac{F_{CB}}{\sqrt{2}} = 0.7071 F_{CB}$$



$$\Sigma F_x = 0$$

$$\Sigma F_y = 0$$

$$\underline{\Sigma F_x = 0} \quad -0.7071 F_{CB} - 15 + 0.3714 F_{CE} = 0$$

Eq 1

$$F_{CE} = 40.388 \text{ kN} + 1.9039 F_{CB}$$

$$\underline{\Sigma F_y = 0} \quad -0.7071 F_{CB} - 0.9285 F_{CE} = 0$$

Eq 2

$$F_{CB} = -1.3131 F_{CE}$$

Sub. Eq 2 into Eq 1

$$F_{CE} = 40.388 + 1.9039(-1.3131 F_{CE})$$

$$F_{CE} = +11.539 \text{ kN (T)}$$

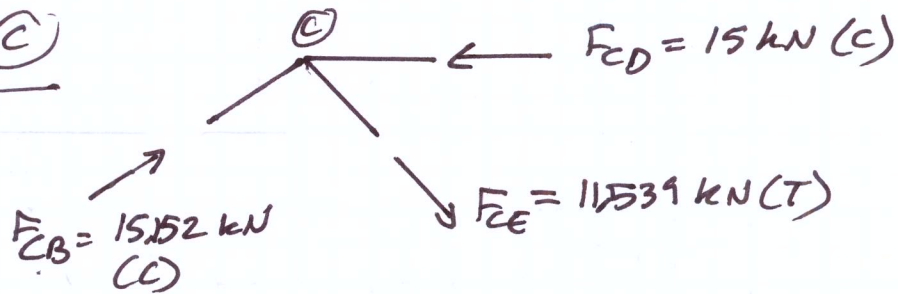
Sub ~~B~~ F_{CE} into Eq 2

$$F_{CB} = -1.3131 F_{CE}$$

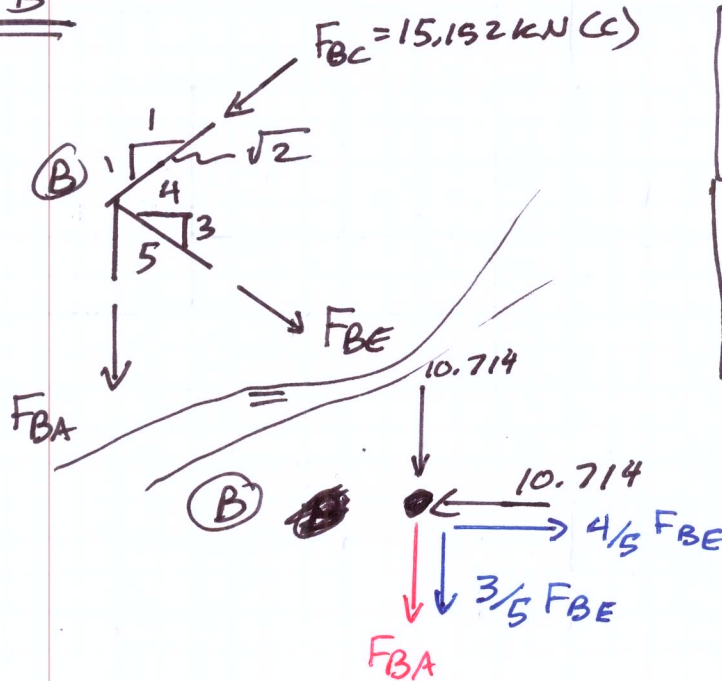
$$F_{CB} = (-1.3131)[11.539]$$

$$F_{CB} = -15.1524 \text{ kN} = 15.1524 \text{ kN (C)}$$

Still at (C)



JOINT B



$$F_{BE_x} = \frac{4}{5} F_{BE}$$

$$F_{BE_y} = \frac{3}{5} F_{BE}$$

$$F_{BC_x} = \frac{F_{BC}}{\sqrt{2}} = 10.714 \text{ kN}$$

$$F_{BC_y} = \frac{F_{BC}}{\sqrt{2}} = 10.714 \text{ kN}$$

$\Sigma F_x = 0$

$$\frac{4}{5} F_{BE} - 10.714 = 0$$

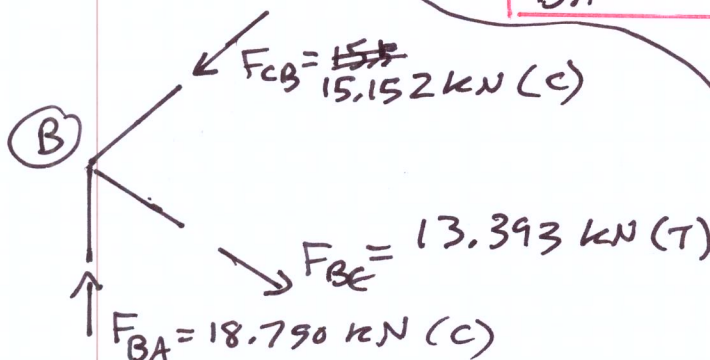
$$F_{BE} = 13.393 \text{ kN (T)}$$

$\Sigma F_y = 0$

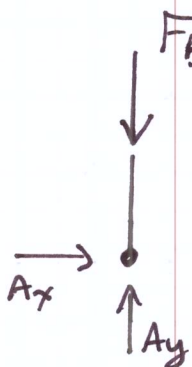
$$-10.714 - F_{BA} - \frac{3}{5} F_{BE} = 0$$

$$-10.714 - F_{BA} - \frac{3}{5} (13.393) = 0$$

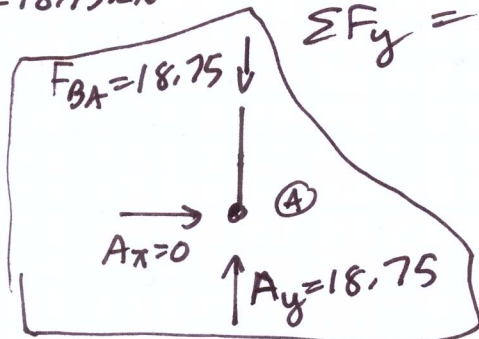
$$F_{BA} = -18.750 \text{ kN} = 18.750 \text{ kN (C)}$$



JOINT A



$F_{BA} = 18.75 \text{ kN}$

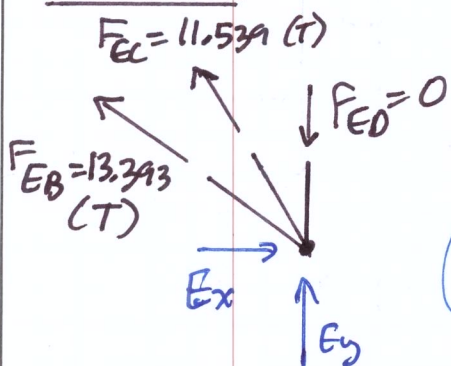


$\sum F_x = 0 \rightarrow A_x = 0$

$\sum F_y = 0 \rightarrow A_y = 18.75 \text{ kN} \uparrow$

From JOINT C

JOINT E



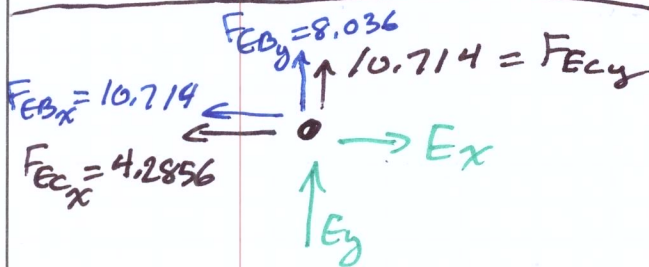
$F_{ECx} = 0.3714 F_{EC} = 4.2856 \text{ kN}$

$F_{ECy} = 0.9285 F_{EC} = 10.714 \text{ kN}$

$F_{EBx} = \frac{4}{5} F_{EB} = 10.714 \text{ kN}$

$F_{EBy} = \frac{3}{5} F_{EB} = 8.036 \text{ kN}$

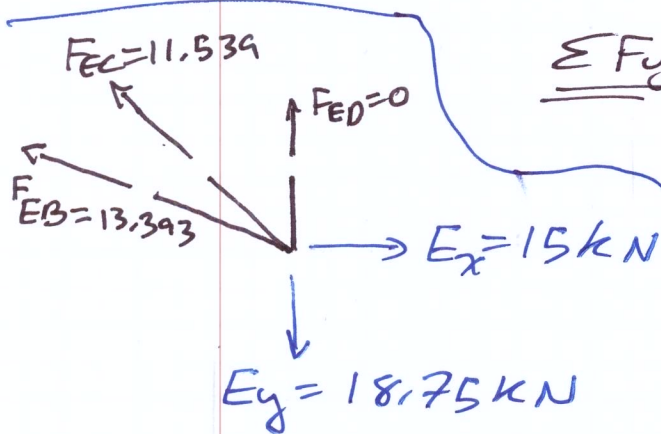
From JOINT B



$\sum F_x = 0$

$E_x - 4.2856 - 10.714 = 0$

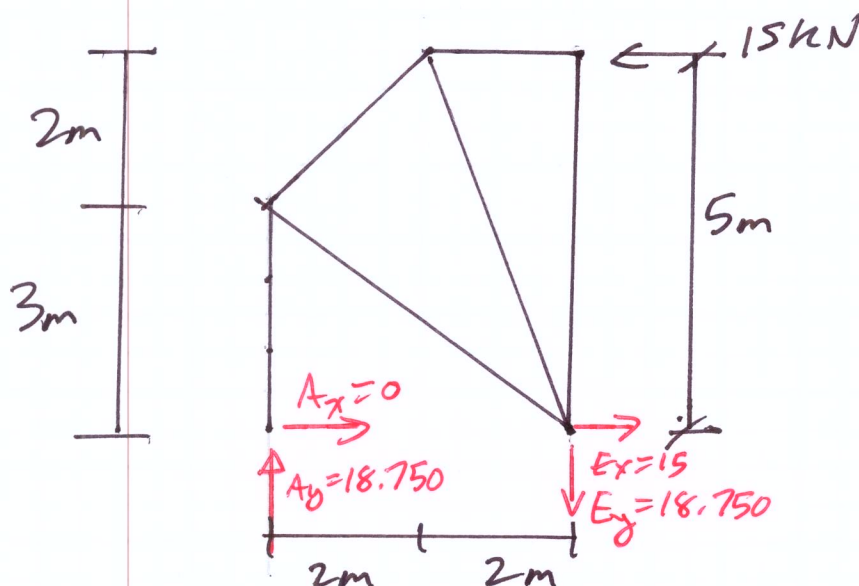
$E_x = 15 \text{ kN}$



$\sum F_y = 0$

$E_y + 10.714 + 8.036 = 0$

$E_y = -18.75 \text{ kN}$



check out answer

- We Analyzed all joints but never took Global Equilibrium

Note: you always have an additional joint or global equilibrium check

$$\underline{\Sigma F_x = 0}$$

$$A_x + E_x - 15 = 0$$

$$0 + 15 - 15 = 0 \quad \checkmark$$

$$\underline{\Sigma F_y = 0}$$

$$A_y - E_y = 0$$

$$15 - 15 = 0 \quad \checkmark$$

$$\underline{\Sigma M_E = 0}$$

$$-A_y \overset{\curvearrowright}{(4)} + 15 \overset{\curvearrowleft}{(5)} = 0$$

$$A_y = +18.75 \text{ kN} \quad \checkmark$$

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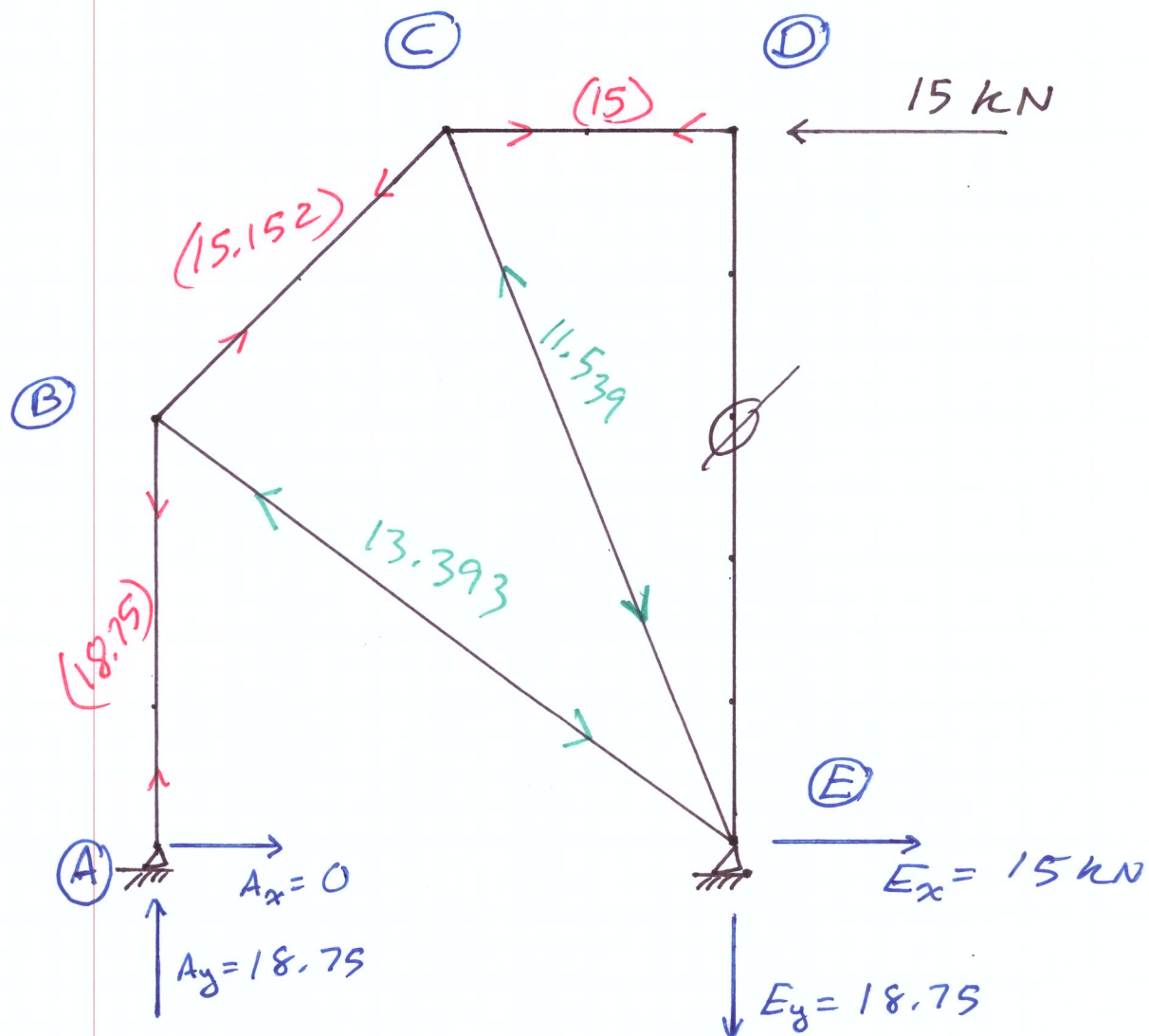
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Problem No.

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ALAN LLOYD

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SUMMARY DIAGRAM**(Compression)**

Tension

 $\phi \rightarrow$ zero-force